

# Large Vibration Amplitudes of Circular Functionally Graded Thin Plates Resting on Winkler Elastic Foundations

El Kaak, Rachid, El Bikri, Khalid, and Benamar, Rhali

**Abstract**—This paper describes a study of geometrically nonlinear free vibration of thin circular functionally graded (CFGP) plates resting on Winkler elastic foundations. The material properties of the functionally graded composites examined here are assumed to be graded smoothly and continuously through the direction of the plate thickness according to a power law and are estimated using the rule of mixture. The theoretical model is based on the classical Plate theory and the Von-Kármán geometrical nonlinearity assumptions. An homogenization procedure (HP) is developed to reduce the problem considered here to that of isotropic homogeneous circular plates resting on Winkler foundation. Hamilton's principle is applied and a multimode approach is derived to calculate the fundamental nonlinear frequency parameters which are found to be in a good agreement with the published results. On the other hand, the influence of the foundation parameters on the nonlinear fundamental frequency has also been analysed.

**Keywords**—Functionally graded materials, nonlinear vibrations, Winkler foundation.

## I. INTRODUCTION

IN recent years, functionally graded materials (FGMs) have gained much popularity as materials to be used in structural components exposed to extremely high-temperature environments such as nuclear reactors and high-speed spacecraft industries. FGMs are composite materials that are microscopically inhomogeneous, and their mechanical properties vary smoothly or continuously from one surface to the other. Typically, these materials are made from a mixture of ceramic and metal, or a combination of different materials. The concept of FGMs was first introduced in Japan in 1984. Since then, it has gained considerable attention [1, 2]. FGMs have various available or potential applications in many fields such as aerospace engineering, electrical engineering, biomedical engineering, and architecture engineering [3, 4].

Thin-plate structures are commonly used in these engineering applications, and they are often subjected to severe dynamic loading, which may result in large vibration amplitudes. When the amplitude of vibration is of the same order of the plate thickness, a significant geometrical nonlinearity is induced and linear models are not sufficient to

predict the dynamic behavior of the plate which may exhibit many new features, such as the amplitude dependence of the frequency and mode shapes on the amplitude of vibration and the jump phenomenon.

Many studies have been devoted to functionally graded plate vibrations in the literature, such as those of Allahverdizadeh, Naei and Nikkha Bahrami [5] who investigated the nonlinear free and forced vibration of thin circular functionally graded plates.

In the present paper, the problem of geometrically nonlinear free vibrations of clamped-clamped CFGP with immovable ends resting on linear and nonlinear Winkler elastic foundation is investigated using Hamilton's principle and spectral analysis. Based on the governing axial equation of the circular plate in which the axial inertia and damping are ignored, an homogenization procedure, previously proposed in [6] in the case of functionally graded beams resting on nonlinear elastic foundations, is used which reduces the problem studied to that of isotropic homogeneous circular plate with effective bending stiffness and axial stiffness parameters.

## II. FUNCTIONALLY GRADED MATERIALS

In this section, we consider a clamped-clamped CFGP having the geometrical characteristics shown in Fig. 1. It is assumed that the CFGP is made of ceramic and metal, and the effective material properties of the CFGP, i.e., Young's modulus  $E$  and mass density  $\rho$ , are functionally graded in the thickness direction according to a function of the volume fractions  $V$  of the constituents.

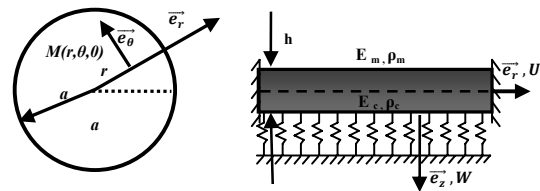


Fig. 1 Geometry of a FG clamped circular plate

According to the rule of mixture, the effective material properties  $P$  can be expressed as:

$$P = P_m V_m + P_c V_c \quad (1)$$

El Kaak, Rachid is with the Université Mohammed V-Souissi ENSET-rabat, LaMIPI, B.P6207, Rabat Instituts, Rabat, Morocco (E-mail: rachidkhint@gmail.com).

El Bikri, Khalid, is with the Université Mohammed V-Souissi ENSET-rabat, LaMIPI, B.P6207, Rabat Instituts, Rabat, Morocco (E-mail: k.elbikri@um5s.net.ma).

Benamar, Rhali is with the Université Mohammed V-Agdal, EMI, LERSIM, Agdal Av. Ibn Sina, Rabat, Morocco (E-mail: rbenamar@emi.ac.ma).

Where subscripts “m” and “c” refer to the metal and ceramic constituents, respectively. A simple power law is considered here to describe the variation of the volume fraction of the metal and the ceramic constituents as follows:

$$V_m = (z/h + 1/2)^n \quad (2)$$

With  $V_m + V_c = 1$   $n$  is a non-negative parameter (power-law exponent) which dictates the material variation profile through the thickness of the plate.

Effective material properties  $P$  of the CFGP such as Young’s modulus ( $E$ ) and mass density ( $\rho$ ) can be determined by substituting (2) into (1), which gives:

$$P(z) = P_m + (P_c - P_m) (z/h + 1/2)^n \quad (3)$$

### III. NONLINEAR FREE VIBRATION ANALYSIS

Consider a fully clamped thin circular plate of a uniform thickness  $h$  and a radius  $a$ . The co-ordinate system is chosen such that the middle plane of the plate coincides with the polar coordinates  $(r, \theta)$ , the origin of the co-ordinate system being at the centre of the plate with the  $z$ -axis downward, as depicted in Fig. 1. The plate is made of a mixture of ceramic and metal. Considering axisymmetric vibrations of the circular plate, the displacements are given in accordance with classical plate theory by:

$$u_r(r, z, t) = U(r, t) - z \partial w(r, t) / (\partial r), u_\theta(r, t) = 0, u_z(r, t) = W(r, t) \quad (4)$$

where  $U$  and  $W$  are the in-plane and out-of-plane displacements of the middle plane point  $(r, \theta, 0)$  respectively, and  $u_r$ ,  $u_\theta$  and  $u_z$  are the displacements along  $r$ ,  $\theta$  and  $z$  directions, respectively.

The non-vanishing components of the strain tensor in the case of large displacements are given by Von-Karman relationships:

$$\{\varepsilon\} = \{\varepsilon^0\} + z\{K\} + \{\lambda^0\} \quad (5)$$

In which  $\{\varepsilon^0\}$ ,  $\{K\}$  and  $\{\lambda^0\}$  are given by:

$$\{\varepsilon^0\} = \begin{bmatrix} \varepsilon_r^0 \\ \varepsilon_\theta^0 \end{bmatrix} = \begin{bmatrix} \partial u / (\partial r) \\ u / (r) \end{bmatrix}, \{K\} = \begin{bmatrix} K_r \\ K_\theta \end{bmatrix} = \begin{bmatrix} -\partial w^2 / (\partial r^2) \\ -1 / (r) \partial w / (\partial r) \end{bmatrix} \quad (6, 7)$$

$$\{\lambda^0\} = \begin{bmatrix} \lambda_r \\ \lambda_\theta \end{bmatrix} = \begin{bmatrix} 1 / (2) (\partial w / (\partial r))^2 \\ 0 \end{bmatrix} \quad (8)$$

For the FGM circular plate shown in Fig. 1, the stress can be expressed as:

$$\{\sigma\} = [Q]\{\varepsilon\} \quad (9)$$

In which  $\{\sigma\} = [\sigma_r \sigma_\theta]^T$  and the terms of the matrix  $[Q]$  can be obtained by the relationships given in reference [7]. The force and moment resultants are defined by:

$$(N_r, N_\theta) = \int_{-h/2}^{h/2} (\sigma_r, \sigma_\theta) dz \quad (10)$$

$$(M_r, M_\theta) = \int_{-h/2}^{h/2} (z \sigma_r, z \sigma_\theta) dz \quad (11)$$

The in-plane forces and bending moments in the plate are given by:

$$\begin{bmatrix} N \\ M \end{bmatrix} = \begin{bmatrix} A & B \\ B & D \end{bmatrix} \begin{bmatrix} \{\varepsilon^0\} + \{\lambda^0\} \\ \{K\} \end{bmatrix} \quad (12)$$

$A$ ,  $B$  and  $D$  are symmetric matrices given by the following equation:

$$(A_{ij}, B_{ij}, D_{ij}) = \int_{-h/2}^{h/2} Q_{ij} (1, z, z^2) dz \quad (13)$$

Here, the  $Q_{ij}$ 's are the reduced stiffness coefficients of the plate. The expression for the bending strain energy  $V_b$ , the membrane strain energy  $V_m$ , the coupling strain energy  $V_c$  and the kinetic energy  $T$  are given by:

$$V_b = \pi \int_0^a D_{11} [(\partial w^2 / (\partial r^2))^2 + 1 / (r)^2 (\partial w / (\partial r))^2 + 2 \nu / (r) \partial w / (\partial r) \partial w^2 / (\partial r^2)] r dr \quad (14)$$

$$V_m = \pi \int_0^a A_{11} [(\partial u / (\partial r))^2 + \partial u / (\partial r) (\partial w / (\partial r))^2] r dr + \pi \int_0^a A_{11} [2 \nu U / (r) \partial u / (\partial r) + \nu U / (r) (\partial w / (\partial r))^2] r dr + \pi \int_0^a A_{11} \left[ \frac{1}{4} (\partial w / (\partial r))^4 + u^2 / (r)^2 \right] r dr \quad (15)$$

$$V_c = \pi \int_0^a -B_{11} [\partial w^2 / (\partial r^2) (\partial w / (\partial r))^2 + \nu / (r) \partial w / (\partial r) (\partial w / (\partial r))^2] r dr \quad (16)$$

and

$$T = \pi I_0 \int_0^a (\partial w / (\partial r))^2 r dr \quad (17)$$

where  $I_0$  is the inertial term given by:

$$I_0 = \int_{-h/2}^{h/2} \rho(z) dz \quad (18)$$

An approximation has been adopted in the present work consisting on neglecting the contribution of the in-plane displacement  $U$  in the membrane strain energy expression. Such an assumption of neglecting the in-plane displacements in the non-linear plate strain energy has been made in Refs.

[8, 9] when calculating the first two non-linear mode shapes of fully clamped rectangular plates. For the first non-linear mode shape, the range of validity of this assumption has been discussed in the light of the experimental and numerical results obtained for the non-linear frequency–amplitude dependence and the non-linear bending stress estimates obtained at large vibration amplitudes. In order to examine the effects of large vibration amplitudes on the membrane stress patterns for clamped circular plates, the assumption introduced above leads to:

$$V_m = \pi (A_{11}/(4)) \int_0^a (\partial w / (\partial r))^4 r dr \quad (19)$$

The strain energy of the elastic foundation  $V_f$  of the CFGP is given by:

$$V_f = \frac{1}{2} \int_0^a \int_0^{2\pi} K_L w^2 r dr d\theta + \frac{1}{4} \int_0^a \int_0^{2\pi} K_{NL} w^4 r dr d\theta \quad (20)$$

where  $K_L$  and  $K_{NL}$  are the linear and the nonlinear Winkler foundation stiffness respectively. For a general parametric study, we use the following non dimensional formulation by putting:

$$r^* = r/(a) , \quad w_i^* = w_i/(h) \quad (21, 22)$$

Applying Hamilton's principle and expanding the displacement  $W$  in the form of a finite series, the following set of nonlinear algebraic equations is obtained:

$$2a_i k_{ir}^* + 3a_i a_j a_k b_{ijk}^* + (8/(\pi)) a_i a_j C_{ijr}^{s*} - 2\omega^{*2} a_i m_{ir}^* = 0 \quad (23)$$

where  $m_{ij}^*$ ,  $k_{ij}^*$ ,  $b_{ijk}^*$  and  $c_{ijk}^{s*}$  stand for the non dimensional mass tensor, the linear rigidity tensor, the fourth order non-linear rigidity tensor and the third order non-linear coupling tensor, respectively, which are defined as:

$$\begin{aligned} k_{ij}^* &= \int_0^1 \left[ \begin{aligned} &(\partial w_i^* / (\partial r^{*2})) (\partial w_j^* / (\partial r^{*2})) \\ &+ (1/(r^{*2})) (\partial w_i^* / (\partial r^*)) (\partial w_j^* / (\partial r^*)) \end{aligned} \right] r^* dr^* \\ &+ \int_0^1 2(v/(r^*)) (\partial w_i^* / (\partial r^*)) (\partial w_j^* / (\partial r^{*2})) r^* dr^* \\ &+ K_L^* \int_0^1 w_i^* w_j^* r^* dr^* \quad (24) \\ C_{ijk}^{s*} &= \beta \int_0^1 [(\partial w_i^* / (\partial r^{*2})) (\partial w_j^* / (\partial r^*)) (\partial w_k^* / (\partial r^*))] r^* dr^* \\ &+ \beta \int_0^1 [(v/(r^*)) (\partial w_i^* / (\partial r^*)) (\partial w_j^* / (\partial r^*)) (\partial w_k^* / (\partial r^*))] r^* dr^* \end{aligned} \quad (25)$$

$$m_{ij}^* = \int_0^1 w_i^* w_j^* r^* dr^* \quad (26)$$

$$\begin{aligned} b_{ijkl}^* &= \alpha \int_0^1 (\partial w_i^* / \partial r^*) (\partial w_j^* / \partial r^*) (\partial w_k^* / \partial r^*) (\partial w_l^* / \partial r^*) r^* dr^* \\ &+ K_{NL}^* \int_0^1 w_i^* w_j^* w_k^* w_l^* r^* dr^* \quad (27) \end{aligned}$$

where  $\alpha$ ,  $\beta$ ,  $K_L^*$ ,  $K_{NL}^*$  and  $K_S^*$  are given by:

$$\alpha = (A_{11} h^2 / (4D_{11})), \quad \beta = (-B_{11} h / (D_{11})) \quad (28, 29)$$

$$K_L^* = (a^4 / (D_{11})) K_L, \quad K_{NL}^* = (2a^4 / (A_{11})) K_{NL} \quad (30, 31)$$

To obtain the nonlinear free response of a clamped-clamped CFGP in the neighborhood of its first resonant frequency, the values of the linear rigidity matrix  $K_{ij}^*$  and the nonlinear geometrical rigidity tensor  $b_{ijkl}^*$  have been calculated using the first six normalized symmetric linear circular plate function,  $w_1^*$ ,  $w_2^*$ , ...,  $w_6^*$ . The functions have been normalized in such a manner that the obtained mass matrix equals the identity matrix.

#### IV. NUMERICAL RESULTS AND DISCUSSIONS

In the problem considered herein, the top surface of the CFGP is ceramic rich ( $E_c=384.43e9$ Pa,  $\nu_c=0.24$ ,  $\rho_c=2370$ Kg/m<sup>3</sup>), whereas the bottom surface of the CFGP is metal rich ( $E_m=201.04e9$ Pa,  $\nu_m=0.3177$ ,  $\rho_m=8166$  Kg/m<sup>3</sup>).

TABLE I  
 FREQUENCY RATIO  $\Omega_{NL}^* / \Omega_L^*$  FOR VARIOUS NON-DIMENSIONAL VIBRATION AMPLITUDES ASSOCIATED WITH THE FIRST MODE SHAPE OF CLAMPED CIRCULAR ISOTROPIC PLATE FOR A POISSON'S RATIO  $\nu=0.28$

$W_{max}^*$	PRESENT WORK 2013	[5] 2008
0.2	1.0108	1.0075
0.4	1.0421	1.0296
0.5	1.0648	1.0459
0.6	1.0916	1.0654
0.8	1.1560	1.1135
1.0	1.2318	1.1724

In table I, the first nonlinear frequency ratios  $\omega_{nl}^* / \omega_l^*$ , calculated in the present work at various vibration amplitudes, is compared with the results obtained in [5]. It is noted that the solution given in the present work overestimates the frequencies of the clamped circular isotropic plate, especially for high values of dimensionless amplitude. This discrepancy is mainly due to the fact that the axial displacements have been neglected in the expression of the axial strain energy.

TABLE II  
 FREQUENCY RATIO  $\Omega_{NL}^*/\Omega_L^*$  FOR VARIOUS NON-DIMENSIONAL VIBRATION  
 AMPLITUDES ASSOCIATED WITH THE FIRST MODE SHAPE OF CLAMPED  
 CIRCULAR FG PLATE

$W_{max}^*$	PRESENT WORK 2013	[5] 2008
0.2	1.0045	1.0068
0.4	1.0176	1.0275
0.5	1.0270	1.0413
0.6	1.0383	1.0586
0.8	1.0655	1.1034
1.0	1.0980	1.1586

The same comparison has been conducted in the case of circular functionally graded plate. As expected, the frequency ratios obtained with the present model are higher than those obtained in [5], especially for large vibration amplitudes for which the contribution of axial displacement becomes significant.

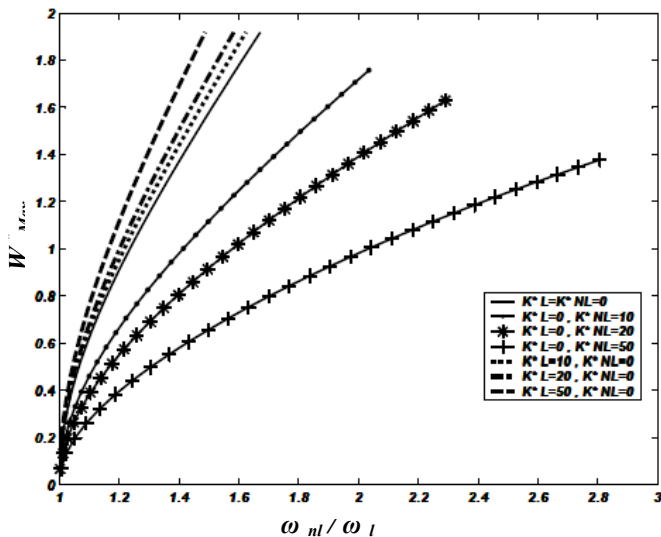


Fig. 2 Effect of the elastic foundation stiffness on the frequency ratio

It can be shown from Fig. 2 that an increase in the value of linear elastic foundation stiffness leads to a decrease in the nonlinear to linear frequency ratio. On the other hand, this ratio enhances with an increase in nonlinear elastic foundation stiffness.

## V. CONCLUSION

The present study deals with the problem of geometrically nonlinear free vibrations of a clamped-clamped CFGP resting on Winkler elastic foundations. A homogenization procedure has been proposed which reduces the problem studied here to that of isotropic homogeneous plate. The main feature of the present contribution is the fact that the existing analytical solutions, numerical techniques and software developed over the years for the nonlinear analysis of isotropic circular plates can be easily used for CFGP case. On the other hand, the influence of the foundation parameters on the nonlinear fundamental frequency has been studied. The effect of the

linear foundation is to soften the nonlinear dynamic behavior of the CFGP, whereas the effect of the nonlinear foundation stiffness is to stiffen the plate response. It's expected in future work to complete the present model by taking into account the contribution of the axial displacement in the axial strain energy expression.

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