

# Development of Rock Engineering System-Based Models for Tunneling Progress Analysis and Evaluation: Case Study of Tailrace Tunnel of Azad Power Plant Project

S. Golmohammadi, M. Noorian Bidgoli

**Abstract**—Tunneling progress is a key parameter in the blasting method of tunneling. Taking measures to enhance tunneling advance can limit the progress distance without a supporting system, subsequently reducing or eliminating the risk of damage. This paper focuses on modeling tunneling progress using three main groups of parameters (tunneling geometry, blasting pattern, and rock mass specifications) based on the Rock Engineering Systems (RES) methodology. In the proposed models, four main effective parameters on tunneling progress are considered as inputs (RMR, Q-system, Specific charge of blasting, Area), with progress as the output. Data from 86 blasts conducted at the tailrace tunnel in the Azad Dam, western Iran, were used to evaluate the progress value for each blast. The results indicated that, for the 86 blasts, the progress of the estimated model aligns mostly with the measured progress. This paper presents a method for building the interaction matrix (statistical base) of the RES model. Additionally, a comparison was made between the results of the new RES-based model and a Multi-Linear Regression (MLR) analysis model. In the RES-based model, the effective parameters are RMR (35.62%), Q (28.6%), q (specific charge of blasting) (20.35%), and A (15.42%), respectively, whereas for MLR analysis, the main parameters are RMR, Q (system), q, and A. These findings confirm the superior performance of the RES-based model over the other proposed models.

**Keywords**—Rock Engineering Systems, tunneling progress, Multi Linear Regression, Specific charge of blasting.

## I. INTRODUCTION

THE estimation of tunneling progress values in rock is a complex and crucial task frequently encountered during tunnel excavation [1]. Developing prediction models has been a primary objective and has been in progress for many years [1]-[3]. In addition to theoretical and empirical models, Artificial Neural Networks (ANN) have been employed to predict the rate of penetration and tunneling progress values [4]-[6]. Fuzzy logic, genetic algorithms, and ANNs have also been utilized to establish predictive models and assign principal parameters in hydrology, mining, and civil engineering applications in recent years [4], [5], [7]-[9]. Most of these models are generated based on experience gained and data compiled from past tunneling projects.

The aim of this study is to evaluate the effect of main

parameters on tunneling progress values using a method called Rock Engineering System (RES).

## II. ROCK ENGINEERING SYSTEM

One of the best strong methods to solving complex engineering problems is RES, which was first introduced by Hudson [10]-[12]. RES method has been widely applied to several engineering problems including environmental studies; regarding the disposal of spent fuel [13], forest ecosystems [14], [15], radioactive waste management [16], [17], traffic-induced air pollution [18], risk of reservoir pollution [19], tunnel boring machine [20], [21] etc. It has also been widely used in most rock mechanics applications such as slope stability [22]-[30], stability analysis of tunnels and underground spaces [20]-[31]-[32] and blasting analysis in rocks [33]-[37].

In RES application to rock engineering, the interaction matrix [10], [12] is the basic analytical tool and a presentational method for characterizing the main parameters and the interaction mechanisms in a RES. In the interaction matrix for a given RES, all parameters influencing the system are arranged along the leading diagonal of the matrix, called the diagonal terms. The effect of each individual parameter on any other parameters is accounted for at the corresponding off-diagonal position, named the off-diagonal terms. The off-diagonal terms are assigned numerical values which explain the influence degree of one parameter on the other parameters. Assignment of these values is called coding the matrix. Different coding methods have been expanded for this purpose. The common coding method is called “expert semi-quantitative” (ESQ). ESQ coding has been used in nearly all previous studies cited above. In this approach, every interaction is assigned a distinct code, effectively symbolizing the impact of a parameter on another within the matrix. Ordinarily, the coding values range from 0 to 4, where 0 signifies no interaction and 4 denotes the maximum level of interaction [21]. The general concept of the influences in a system is defined by the interaction matrix, which is shown in Fig. 1 [10], [12]. In this matrix, rock mechanics values are the primary parameters instead of numbers. Here, the influence of ‘A’ on ‘B’ is not the same as the influence of B on A, which means the matrix is asymmetric [38]. Thus, it is important to

S. Golmohammadi is PhD candidate of Mining Engineering, Faculty of engineering, University of Kashan, Kashan, Iran.

M. Noorian Bidgoli was with Department of Mining Engineering, Faculty of engineering, University of Kashan, Kashan, Iran (corresponding author, phone: 00989133629242; e-mail: noriyan.kashanu@gmail.com)

put the parameter interactions in clockwise direction in the matrix. In the interaction matrix, the sum of arrow is called the ‘‘cause’’ value ( $C_{pi} = \sum_j I_{ij}$ ) and the sum of a column is the ‘‘effect’’ value ( $E_{pj} = \sum_i I_{ij}$ ). Represented as coordinates (C, E) corresponding to a specific parameter, these values can be plotted in a cause-and-effect space, creating what is known as a C–E plot. The interactive intensity of each parameter is expressed as the sum of its C and E values (C+E), serving as an indicator of the parameter's significance within the system [40]. The percentage value of (C+E) can be used as the parameter's weighting factor ( $\alpha_i$ ) as follows:

$$\alpha_i = \frac{(C_i + E_i)}{\left(\sum_i C_i + \sum_i E_i\right)} \times 100 \quad (1)$$

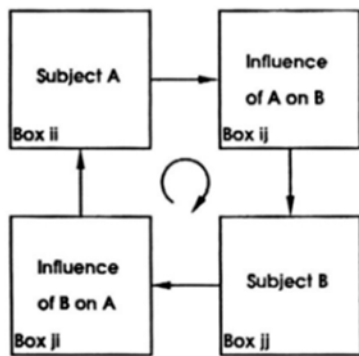


Fig. 1 General illustration of interaction matrix with two factors [10]

In order to use RES model in this project, a database comprising rock mass rating (RMR), Q (system) of rock mass, tunneling geometry specifications (Area), and blasting specification (specific charge) data has been established by collecting field data from completed tunnels in the Azad Dam,

Iran. Subsequently, utilizing the established dataset, both the MLR and RES models are developed to assess the tunneling progress value under these geological conditions.

### III. CASE STUDY

The Azad Pumped Storage Power Plant (PSPP) is situated in the western part of Iran at coordinates 35.21 N and 46.34 E along the Kumasi River. This project is designed to harness hydraulic potentiality by utilizing a pumping system during the low load conditions of the power supply network. Subsequently, it generates electricity through turbines and generators during peak load conditions of the network. The Azad PSPP consists of two reservoirs and a power plant. The lower reservoir is the Azad Dam reservoir, while the upper reservoir is created through excavation at an elevation of 1,900 m [39]. The Azad power plant site is positioned in the Sanandaj–Sirjan formation, characterized by the alternation of sandstone, schist, phyllite, and conglomerate [41].

To investigate the geological and hydrological conditions of the reservoir, six exploratory boreholes were drilled to various depths, and permeability tests were conducted in each of them. The tailrace tunnel, known as Payab, in the Azad Dam was specifically studied in the field to establish a database for developing the RES model to evaluate the main parameters affecting tunneling progress as shown in Fig. 2. The tailrace tunnel, constructed in 2017, spanned approximately 660 meters in length with a 40 square meter area. To facilitate excavation, drilling and blasting methods were employed. The rock strength in the mentioned sites ranged between 90 and 120 MPa, drilling and blasting are necessary for construction purposes. During the drilling process, blast holes with a diameter of 51 mm and a depth of 3.5 meters were employed. In the blasting process, dynamite was used as the primary explosive. Additionally, the blast holes were stemmed with fine gravels.

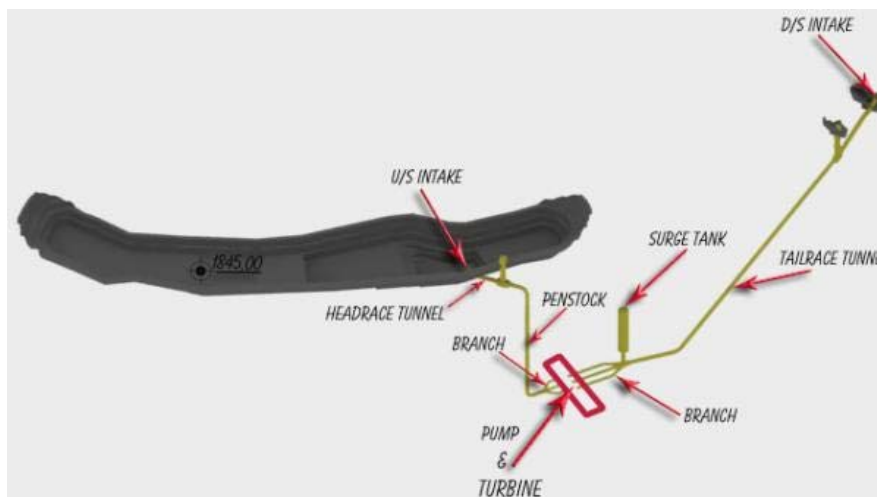


Fig. 2 Schematic layout of Azad PSPP structures

In this research, four main parameters were chosen for analysis, as RMR and Q encompass crucial effective parameters that can distinctly describe rock mass properties, as illustrated

in Table I. The significance of these parameters in the modeling process was explored by examining the correlations between individual independent variables and the measured progress.

The coefficient of determination ( $R^2$ ) was employed as an indicator of correlation strength. The  $R^2$  values for independent variables versus Pr (Tunneling progress) are outlined in Table II. Consequently, to proceed with further statistical analysis and the development of a prediction model, these four independent variables were selected.

TABLE I  
THE RANGE OF USED PARAMETERS IN THE PRESENT PAPER FOR PREDICTING TUNNELING PROGRESS

Parameter	Unit	Min	Max
RMR	percentage	32	57
Tunneling Quality Index (Q)		0.72	3.5
Blasting specific charge (q)	kg/m <sup>3</sup>	0.5	4
Tunnel face area (A)	m <sup>2</sup>	36.7	54.53
Tunneling progress (Pr)	m	0.35	4.65

TABLE II  
RELATIONS BETWEEN INDIVIDUAL INDEPENDENT VARIABLES AND TUNNELING PROGRESS (Pr) FOR 86 BLASTS, AZAD TAILRACE TUNNEL

Independent variables	Regression	R <sup>2</sup>	RMSE
RMR	$P_r = -0.0054RMR^2 + 0.5741RMR - 11.709$	0.6047	0.4639
Tunneling Quality Index (Q)	$P_r = 0.1598Q^2 - 1.3139Q + 4.4917$	0.3341	0.6021
blasting specific charge (q)	$P_r = -0.1853q^2 + 0.0064q + 3.7636$	0.4129	0.5654
Tunnel face area (A)	$P_r = 0.0095A^2 - 0.859A + 22.127$	0.0854	0.7057
Dependent variable (Pr)			

#### A. Evaluating Tunneling Progress

In the current paper, MLR and RES methodologies were employed to formulate a precise and acceptable equation for evaluating tunneling progress resulting from blasting. For the development of both MLR and RES models, four influential parameters affecting progress, namely RMR, Q, q (specific charge), and Area, were adopted as inputs, with progress designated as the output parameter.

#### B. Prediction of Progress by MLR

The MLR is one of the most famous methods to fit a linear equation between one or more independent parameters and one dependent parameter. This method is widely developed to predict some problems in the fields of rock mechanics and geotechnical engineering.

TABLE III  
VARIABLE PARAMETER: RMR

Par	Min	Max	RMR <sub>10%</sub>	RMR <sub>20%</sub>	RMR <sub>30%</sub>	RMR <sub>40%</sub>	RMR <sub>50%</sub>	RMR <sub>60%</sub>	RMR <sub>70%</sub>
Pt	1	4.65	2.02	2.19	2.36	2.53	2.69	2.86	3.03
RMR	32	57	36.3	39.6	42.9	46.2	49.5	52.8	56.1
Q	0.72	3.5	2.5	2.5	2.5	2.5	2.5	2.5	2.5
q	0.505	4.007	1.8	1.8	1.8	1.8	1.8	1.8	1.8
A	36.7	54.53	42.1	42.1	42.1	42.1	42.1	42.1	42.1
Pt			9.08%	18.16%	27.23%	36.31%	45.39%	54.47%	63.55%

#### IV. RES BASED MODEL TO EVALUATE TUNNELING PROGRESS

The principles of RES were incorporated into the methodology, as explained in the introduction section. In this paper, a similar approach is adopted to formulate a model predicting tunneling progress, considering the main parameters

Typically, the MLR model can be formulated as follows:

$$Y = P_0 + P_1X_1 + \dots + P_nX_n \quad (2)$$

where  $X_i$  ( $i = 1, \dots, n$ ) and  $Y$  define independent and dependent parameters, respectively. In addition,  $P_i$  ( $i = 0, 1, \dots, n$ ) defines regression coefficients. Taking into consideration the established datasets, (3) was created by using SPSS v16 software:

$$P_r = 2.84 + 0.051RMR - 0.39Q - 0.48q - 0.02A \quad (3)$$

where RMR, Q, q and A represent RMR and Q-value of Barton, Specific charge in terms of kg/m<sup>3</sup>, Area and Pr is tunneling progress in terms of meter respectively. As shown in Fig. 3 measured value and predicted progress value plotted and correlation value  $R^2 = 0.736$  is acceptable.

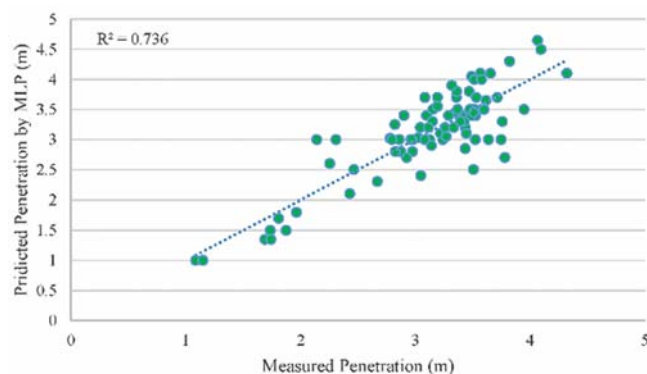


Fig. 3 The measured Progress versus predicted Progress by MLR

#### C. Sensitivity Analysis

To examine the impact of variations in each parameter on tunneling progress, the value of each parameter was individually altered, and the effect of each was computed, as presented in Tables III-VI. The results indicate that RMR, Q, q, and A affect the system in the respective order, as illustrated in Fig. 5. In this paper, the MLR equations between parameters are linear, and the sensitivity analysis ultimately reveals the influential parameters.

outlined in Table I. Defining the model involves three primary phases. The first step is to identify the parameters responsible for potential risks during progress, analyze their behavior, and evaluate the significance (weight) each holds in the overall risk conditions.

**A. Interaction Matrix**

Having constructed an interaction matrix, the next step is to 'code' the off-diagonal components in order to show their importance or to enable mathematical utilization of the matrix. In this paper, a method was used to create interaction matrix, in this method relation between each main parameter indicated with coefficient of determination  $R^2$ . Main parameters located in main diagonal of matrix and relation between parameters located in off-diagonal as coefficient of determinations. The benefit of this method is statistical base and not expert judgment.

TABLE IV  
 VARIABLE PARAMETER: Q (Q SYSTEM)

Par	Min	Max	Q <sub>10%</sub>	Q <sub>20%</sub>	Q <sub>30%</sub>	Q <sub>40%</sub>
Pt	1.00	4.65	1.75	1.66	1.56	1.46
RMR	32.00	57.00	33	33	33	33
Q	0.72	3.50	2.75	3.00	3.25	3.50
q	0.505	4.007	1.8	1.8	1.8	1.8
A	36.70	54.53	42.1	42.1	42.1	42.1
Pt			5.29%	10.59%	15.88%	21.18%

TABLE V  
 VARIABLE PARAMETER: q (SPECIFIC CHARGE OF BLASTING)

Par	Min	Max	q <sub>10%</sub>	q <sub>20%</sub>	q <sub>30%</sub>	q <sub>40%</sub>	q <sub>50%</sub>	q <sub>60%</sub>	q <sub>70%</sub>	q <sub>80%</sub>
Pt	1.00	4.65	1.766	1.679	1.592	1.504	1.417	1.329	1.242	1.154
RMR	32.00	57.00	33	33	33	33	33	33	33	33
Q	0.72	3.50	2.502	2.502	2.502	2.502	2.502	2.502	2.502	2.502
q	0.505	4.007	1.98	2.16	2.34	2.52	2.7	2.88	3.06	3.24
A	36.70	54.53	42.1	42.1	42.1	42.1	42.1	42.1	42.1	42.1
Pt			4.72%	9.44%	14.16%	18.87%	23.59%	28.31%	33.03%	37.75%

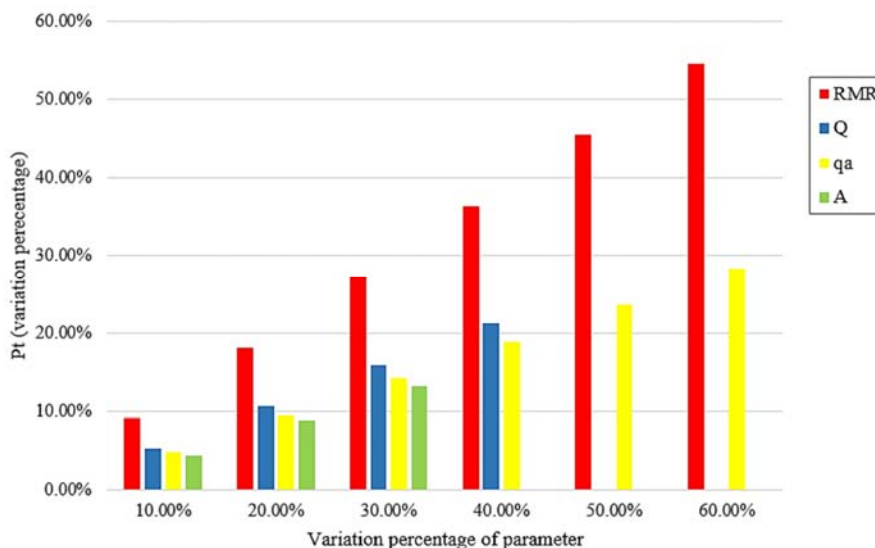


Fig. 4 Effect of parameters variation on tunneling progress

TABLE VI  
 VARIABLE PARAMETER: A

Par	Min	Max	A <sub>10%</sub>	A <sub>20%</sub>	A <sub>30%</sub>
Pt	1.00	4.650	1.772	1.691	1.609
RMR	32.00	57.000	33	33	33
Q	0.72	3.500	2.502	2.502	2.502
q	0.505	4.007	1.8	1.8	1.8
A	36.70	54.530	46.31	50.52	54.73
			4.40%	8.80%	13.20%

**B. Polynomial Regression**

A polynomial regression stands as a method within regression analysis where the correlation between the independent variable x and the dependent variable y is depicted through an nth-degree polynomial. It serves as an expansion of linear regression, which solely deals with linear associations between variables.

Within polynomial regression, the x and y relationship finds expression through a polynomial equation, capable of capturing intricate relationships, encompassing curvature and other non-linear trends. The polynomial equation's degree determines the extent of complexity within the relationship. The most straightforward manifestation of polynomial regression is a second-degree polynomial, also known as a quadratic equation. The relationship between each main parameter is indicated by the coefficient of determination ( $R^2$ ) was shown in Table VII.

In addition to polynomial method of regression, linear method and power method were calculated and results were compared with each other. Parameters' effects on progress in three methods was shown in Table X. Finally, all of the methods illustrate that RMR, Q, q, and A have high effect respectively. In this paper polynomial trend line was used to coding the interaction matrix because it is real in nature and matrix is not

symmetric and relations between parameters were evaluated carefully. Generally, all of the methods show same trend and efficacy and its logical values.

TABLE VII

ALL RELATIONS AMONG MAIN PARAMETERS IN POLYNOMIAL MODE

Dependent Par	Independent Par	Equation	R <sup>2</sup>
P <sub>t</sub>	RMR	$P_t = -0.0054RMR^2 + 0.5741RMR - 11.709$	0.6047
P <sub>t</sub>	Q	$P_t = 0.1598Q^2 - 1.3139Q + 4.4917$	0.3341
P <sub>t</sub>	q	$P_t = -0.1853q^2 + 0.0064q + 3.7636$	0.4129
P <sub>t</sub>	A	$P_t = 0.0095A^2 - 0.859A + 22.127$	0.0854
RMR	P <sub>t</sub>	$RMR = -0.6734P_t^2 + 9.5418P_t + 23.622$	0.5097
Q	P <sub>t</sub>	$Q = 0.1543P_t^2 - 1.2964P_t + 3.7443$	0.3757
q	P <sub>t</sub>	$q = 0.0331P_t^2 - 0.7064P_t + 3.6427$	0.3832
A	P <sub>t</sub>	$A = 0.9854P_t^2 - 6.0401P_t + 47.611$	0.1333
RMR	Q	$RMR = -3.8022Q^2 + 9.3998Q + 41.856$	0.2239
RMR	q	$RMR = -1.0457q^2 + 0.4724q + 49.34$	0.1597
RMR	A	$RMR = 0.1004A^2 - 8.9657A + 242.71$	0.1225
Q	RMR	$Q = 0.0072RMR^2 - 0.6859RMR + 17.386$	0.4645
q	RMR	$q = 0.0018RMR^2 - 0.2034RMR + 7.2434$	0.1611
A	RMR	$A = 0.0215RMR^2 - 2.0117RMR + 85.095$	0.1706
Q	q	$Q = 0.0377q^2 + 0.0252q + 1.1048$	0.03926
Q	A	$Q = -0.0101A^2 + 0.9445A - 20.131$	0.2436
q	Q	$q = -0.3657Q^2 + 1.5191Q + 0.5458$	0.09682
A	Q	$A = 1.0222Q^2 - 2.0516Q + 39.508$	0.1627
A	q	$A = 0.289q^2 - 1.8972q + 41.228$	0.04195
q	A	$q = 0.0075A^2 - 0.6897A + 17.172$	0.08591

TABLE VIII  
INTERACTION MATRIX BASE R<sup>2</sup>

RMR	0.4645	0.1611	0.1706	0.6047	1.4009
0.2239	Q	0.09682	0.1627	0.3341	0.81752
0.1597	0.03926	q	0.04195	0.4129	0.65381
0.1225	0.2436	0.08591	A	0.0854	0.53741
0.5097	0.3757	0.3832	0.1333	P <sub>t</sub>	1.4009
1.0158	1.12306	0.72703	0.50855	1.0158	

TABLE IX  
WEIGHTING OF THE PRINCIPAL PARAMETERS IN TUNNELING PROGRESS

Par	C	E	C+E	C-E	ai(%)
RMR	1.4009	1.0158	2.4167	0.3851	35.62
Q	0.81752	1.12306	1.94058	-0.30554	28.60
q	0.65381	0.72703	1.38084	-0.07322	20.35
A	0.53741	0.50855	1.04596	0.02886	15.42
sum	3.40964	3.37444	6.78408	0.0352	100

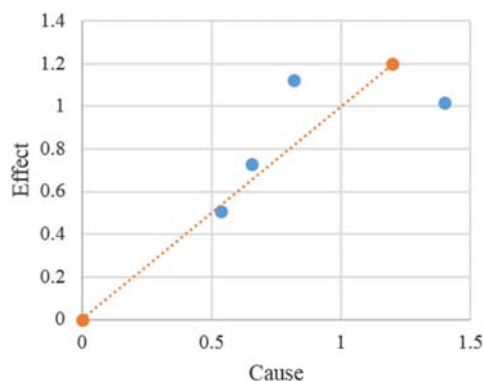


Fig. 5 Cause and effect diagram

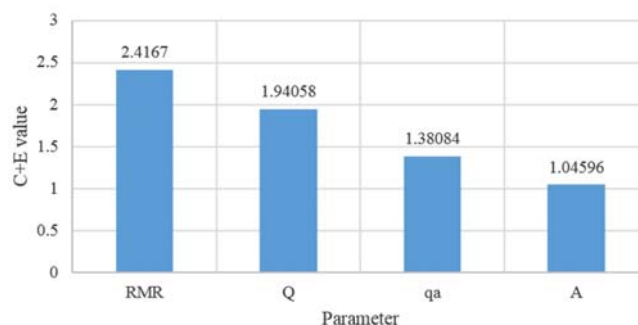


Fig. 6 C+E Value versus main parameters

TABLE X  
PARAMETERS EFFECTS ON PROGRESS IN THREE METHODS

No	Par	ai(%) Polynomial	ai(%) Liner	ai(%) Power
1	RMR	35.62	35.97	38.22
2	Q	28.60	28.32	29.06
3	q	20.35	26.01	20.76
4	A	15.42	9.70	11.97
	sum	100.00	100.00	100.00

## V. CONCLUSION

In this paper, the main effective parameters on tunneling progress were investigated using two main methods, MLR and RES, and it was demonstrated that both methods yield similar results. Notably, a method for coding the interaction matrix was presented. In this approach, the interaction matrix was coded based on the R-square value and validated through analytical approaches. The R-square interaction matrix was constructed using three regression methods (polynomial, linear, and power), and the results were compared. The comparison revealed consistent significance and importance of parameters across the different regression methods. One notable advantage of this method is that it eliminates the need for expert values of parameters in the interaction matrix. Additionally, in the polynomial regression method, the interaction matrix is not symmetric, making it more realistic than conventional methods. Finally, the method was applied in a case study involving the Azad Tailrace Tunnel. The results showcased the significance values of parameters, highlighting that RMR holds high significance and indicating a decrease in the significance of Q (system), q (specific charge), and A (Area), respectively.

## REFERENCES

- [1] Yagiz, S. and H. Karahan, Prediction of hard rock TBM penetration rate using particle swarm optimization. *International Journal of Rock Mechanics and Mining Sciences*, 2011. 48(3): p. 427-433.
- [2] Yagiz, S., Utilizing rock mass properties for predicting TBM performance in hard rock condition. *Tunnelling and Underground Space Technology*, 2008. 23(3): p. 326-339.
- [3] Yagiz, S., et al., Application of two non-linear prediction tools to the estimation of tunnel boring machine performance. *Engineering Applications of Artificial Intelligence*, 2009. 22(4-5): p. 808-814.
- [4] Grima, M.A., P. Bruines, and P. Verhoef, Modeling tunnel boring machine performance by neuro-fuzzy methods. *Tunnelling and underground space technology*, 2000. 15(3): p. 259-269.
- [5] Okubo, S., K. Fukui, and W. Chen, Expert system for applicability of tunnel boring machines in Japan. *Rock Mechanics and Rock Engineering*, 2003. 36(4): p. 305-322.
- [6] Yagiz, S., J. Rostami, and L. Ozdemir. Recommended rock testing

- methods for predicting TBM performance: Focus on the CSM and NTNU Models. in ISRM International Symposium-5th Asian Rock Mechanics Symposium. 2008. International Society for Rock Mechanics and Rock Engineering.
- [7] Benardos, A. and D. Kaliampakos, Modelling TBM performance with artificial neural networks. *Tunnelling and Underground Space Technology*, 2004. 19(6): p. 597-605.
- [8] Gokceoglu, C. and K. Zorlu, A fuzzy model to predict the uniaxial compressive strength and the modulus of elasticity of a problematic rock. *Engineering Applications of Artificial Intelligence*, 2004. 17(1): p. 61-72.
- [9] Jalalifar, H., et al., Application of the adaptive neuro-fuzzy inference system for prediction of a rock engineering classification system. *Computers and Geotechnics*, 2011. 38(6): p. 783-790.
- [10] Hudson, J., *Rock engineering systems. Theory and practice*. 1992.
- [11] Hudson, J., A review of Rock Engineering Systems (RES) applications over the last 20 years, in *Rock Characterisation, Modelling and Engineering Design Methods*. 2013, Taylor & Francis. p. 419-424.
- [12] Hudson, J. and J. Harrison, A new approach to studying complete rock engineering problems. *Quarterly Journal of Engineering Geology and Hydrogeology*, 1992. 25(2): p. 93-105.
- [13] Skagius, K., M. Wiborgh, and A. Stroem, The use of interaction matrices for identification, structuring and ranking of FEPs in a repository system. Application on the far-field of a deep geological repository for spent fuel. 1995, Swedish Nuclear Fuel and Waste Management Co.
- [14] Avila, R. and L. Moberg, A systematic approach to the migration of <sup>137</sup>Cs in forest ecosystems using interaction matrices. *Journal of environmental radioactivity*, 1999. 45(3): p. 271-282.
- [15] Velasco, H., et al., Interaction matrices as a first step toward a general model of radionuclide cycling: application to the <sup>137</sup>Cs behavior in a grassland ecosystem. *Journal of radioanalytical and nuclear chemistry*, 2006. 268(3): p. 503-509.
- [16] Agüero, A., et al., Application of the Spanish methodological approach for biosphere assessment to a generic high-level waste disposal site. *Science of the total environment*, 2008. 403(1-3): p. 34-58.
- [17] van Dorp, F., et al., Biosphere modelling for the assessment of radioactive waste repositories; the development of a common basis by the BIOMOVs II reference biospheres working group. *Journal of environmental radioactivity*, 1999. 42(2-3): p. 225-236.
- [18] Mavroulidou, M., S.J. Hughes, and E.E. Hellawell, A qualitative tool combining an interaction matrix and a GIS to map vulnerability to traffic induced air pollution. *Journal of Environmental Management*, 2004. 70(4): p. 283-289.
- [19] Condor, J. and K. Asghari, An alternative theoretical methodology for monitoring the risks of CO<sub>2</sub> leakage from wellbores. *Energy Procedia*, 2009. 1(1): p. 2599-2605.
- [20] Benardos, A. and D. Kaliampakos, A methodology for assessing geotechnical hazards for TBM tunnelling—illustrated by the Athens Metro, Greece. *International Journal of Rock Mechanics and Mining Sciences*, 2004. 41(6): p. 987-999.
- [21] Frough, O. and S.R. Torabi, An application of rock engineering systems for estimating TBM downtimes. *Engineering Geology*, 2013. 157: p. 112-123.
- [22] Budetta, P., A. Santo, and F. Vivencio, Landslide hazard mapping along the coastline of the Cilento region (Italy) by means of a GIS-based parameter rating approach. *Geomorphology*, 2008. 94(3-4): p. 340-352.
- [23] Castaldini, D., et al., An integrated approach for analysing earthquake-induced surface effects: a case study from the Northern Apennines, Italy. *Journal of Geodynamics*, 1998. 26(2-4): p. 413-441.
- [24] Ceryan, N. and S. Ceryan, An application of the interaction matrices method for slope failure susceptibility zoning: Dogankent settlement area (Giresun, NE Turkey). *Bulletin of Engineering Geology and the Environment*, 2008. 67(3): p. 375-385.
- [25] KhaloKakaie, R. and M.Z. Naghadehi, The assessment of rock slope instability along the Khosh-Yeylagh Main Road (Iran) using a systems approach. *Environmental earth sciences*, 2012. 67(3): p. 665-682.
- [26] Mazzoccola, D. and J. Hudson, A comprehensive method of rock mass characterization for indicating natural slope instability. *Quarterly Journal of Engineering Geology and Hydrogeology*, 1996. 29(1): p. 37-56.
- [27] Naghadehi, M.Z., et al., A probabilistic systems methodology to analyze the importance of factors affecting the stability of rock slopes. *Engineering Geology*, 2011. 118(3-4): p. 82-92.
- [28] Rozos, D., et al., An implementation of rock engineering system for ranking the instability potential of natural slopes in Greek territory. An application in Karditsa County. *Landslides*, 2008. 5(3): p. 261-270.
- [29] Shang, Y., H.-D. Park, and Z. Yang, Engineering geological zonation using interaction matrix of geological factors: an example from one section of Sichuan-Tibet Highway. *Geosciences Journal*, 2005. 9(4): p. 375.
- [30] Zhang, L., et al., An application of the rock engineering systems (RES) methodology in rockfall hazard assessment on the Chengdu-Lhasa highway, China. *International Journal of Rock Mechanics and Mining Sciences*, 2004. 41: p. 833-838.
- [31] Shang, Y., et al., Retrospective case example using a comprehensive suitability index (CSI) for siting the Shisan-Ling power station, China. *International Journal of Rock Mechanics and Mining Sciences*, 2000. 37(5): p. 839-853.
- [32] Shin, H.-S., et al., Methodology for quantitative hazard assessment for tunnel collapses based on case histories in Korea. *International Journal of Rock Mechanics and Mining Sciences*, 2009. 46(6): p. 1072-1087.
- [33] Andrieux, P. and J. Hadjigeorgiou, The destressability index methodology for the assessment of the likelihood of success of a large-scale confined destress blast in an underground mine pillar. *International journal of rock mechanics and mining sciences*, 2008. 45(3): p. 407-421.
- [34] Latham, J.-P. and P. Lu, Development of an assessment system for the blastability of rock masses. *International Journal of Rock Mechanics and Mining Sciences*, 1999. 36(1): p. 41-55.
- [35] Faramarzi, F., M.E. Farsangi, and H. Mansouri, An RES-based model for risk assessment and prediction of backbreak in bench blasting. *Rock mechanics and rock engineering*, 2013. 46(4): p. 877-887.
- [36] Faramarzi, F., H. Mansouri, and M.A.E. Farsangi, Development of rock engineering systems-based models for flyrock risk analysis and prediction of flyrock distance in surface blasting. *Rock mechanics and rock engineering*, 2014. 47(4): p. 1291-1306.
- [37] Faramarzi, F., H. Mansouri, and M.E. Farsangi, A rock engineering systems based model to predict rock fragmentation by blasting. *International Journal of Rock Mechanics and Mining Sciences*, 2013. 60: p. 82-94.
- [38] Hudson, J.A. and J.P. Harrison, *Engineering rock mechanics: an introduction to the principles*. 2000: Elsevier.
- [39] Aalianvari, A., M.M. Tehrani, and S. Soltanimoammadi, Application of geostatistical methods to estimation of water flow from upper reservoir of Azad pumped storage power plant. *Arabian Journal of Geosciences*, 2013. 6(7): p. 2571-2579.
- [40] Fattahi, H. (2018). "An estimation of required rotational torque to operate horizontal directional drilling using rock engineering systems." *Journal of Petroleum Science and Technology* 8(1): 82.
- [41] Aghanabati, A., *Geology of Iran*. 2004: Geological survey of Iran.